STRUCTURAL SPECIFICATIONS

1: GENERAL REQUIREMENTS
CODE REQUIREMENTS - THE BUILDING STRUCTURE IS DESIGNED IN ACCORDANCE W/ THE EIGHTH EDITION (2023) OF THE FLORIDA BUILDING CODE. FOLLOW ALL APPLICABLE PROVISIONS FOR ALL PHASES OF CONSTRUCTION.

ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE IDENTICAL AND SHALL BE CONSTRUCTED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE IN THE WORK EXCEPT WHERE A DIFFERENT DETAIL IS SHOW.

THE CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE JOB SITE PRIOR TO COMMENCING WORK. THE CONTRACTOR SHALL REPORT ALL DISCREPANCIES BETWEEN THE DRAWING AND EXISTING CONDITIONS TO THE DESIGNER PRIOR TO BIDDING AND AGAIN PRIOR TO COMMENCING WORK.

ALL BOLTS NUTS, WASHERS, STRAPS AND FASTENERS INCLUDING NAILS, SHALL BE HOT DIP GALV. CONT. ANCHORAGE SHALL BE PROVIDED BETWEEN ALL TRUSSES, WALL SECTIONS, BEAMS, POSTS, FOOTINGS W/ THE USE OF STRAPS AND CONNECTORS AS SPECIFIED HEREIN, EXCEPT FOR INT. FRAMING, UNLESS NOTED OTHERWISE.

IF ALUM. IS TO BE USED IN ANY AREAS ALL FASTENERS IN CONTACT SHALL BE ALUM. ALSO, OR A BARRIER SHALL BE PROVIDED TO PREVENT GALVANIC ACTION BETWEEN DISSIMILAR METALS.

ALL OPENINGS AND OPENING LOCATIONS ARE BASED IN NOMINAL MODULAR SIZES. DIMENSIONS MAY VARY DUE TO FURRING, BLOCKING, TRUSS ALIGNMENT, LOADING OR OTHER CONDITIONS NECESSARY TO COMPLETE CONSTRUCTION. DIMENSIONS MAY NOT BE IDENTICAL TO THE ACTUAL REQUIREMENT DUE TO THE MFGR. AND/OR SERIES THAT WAS SELECTED.

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION W/ JOB SPECIFICATIONS AND ARCHITECTURAL , MECHANICAL, ELECTRICAL, PLUMBING AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

CONTRACTOR SHALL LOCATE ALL BURIED UTILITIES PRIOR TO EXCAVATION FOR BUILDING FOUNDATIONS. THE STRUCTURAL ENGINEER SHALL BE NOTIFIED OF POTENTIAL CONFLICTS BETWEEN FOUNDATIONS AND BURIED UTILITIES.

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER BUILDING IS COMPLETE. IT IS THE CONTRACTORS SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMP. BRACING, GUYS OR TIE DOWNS, AND SHALL BE REMOVED ONCE COMPLETE AND SHALL REMAIN THE PROPERTY OF THE CONTRACTOR.

2: NOT USED

3: FOUNDATIONS
FOUNDATIONS ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2,500 P.S.F. ON COMPACTED FILL. BEFORE CONSTRUCTION COMMENCES, SOIL BEARING CAPACITY SHALL BE VERIFIED BY A SUBSURFACE GEOTECHNICAL INVESTIGATION, AS WELL AS FIELD AND LABORATORY TESTS PERFORMED BY A CERTIFIED TESTING LABORATORY, WHO'S REPORT SHALL INCLUDE ANALYSIS AND RECOMMENDATIONS FOR SITE PREPARATION IN ORDER TO BEAR THE FOUNDATION LOADS. ABOVE REPORT SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW BEFORE FOUNDATION CONSTRUCTION BEGINS.

ALL CONC. SLABS ON GRADE SHALL BE 4" THICK AND REINF. W/ 6x6 W1.4/1.4 W.W.M. OR FIBERMESH REINFORCING OVER VAPOR BARRIER AS SPECIFIED, U.N.O.

WHERE FIBERMESH REINFORCING IS USED CONC. MUST BE MIXED IN ACCORDANCE W/ MFGR'S RECOMMENDATIONS.

CENTER ALL FOOTINGS AND PIERS UNDER COLUMNS ABOVE, UNLESS SPECIFICALLY DIMENSIONED OTHERWISE.

PROVIDE TERMITE PROTECTION OVER WELL COMPACTED GRANULAR FILL (SEE ARCHITECTURAL OUTLINE SPECIFICATIONS)

ALL FOOTINGS TO EXTEND BELOW GRADE A MIN. OF 16" AT BEARING WALLS. INTERIOR BEARING FOOTINGS 6" INTO CONSTRUCTION FILL U.N.O.

FILL UNDER CONC. SLABS SHALL BE CLEAN SAND OR ROCK AND FREE OF DEBRIS AND OTHER DELETERIOUS MATERIAL. FILL SHALL BE COMPACTED TO A DENSITY OF AT LEAST 95% OF MODIFIED PROCTOR MAX. DRY DENSITY (A.S.T.M. D1557)

WELDED WIRE MESH SHALL CONFORM TO A.S.T.M.-A-185, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE W/ THE TYP. PLACE DETAILS OF A.C.I. STANDARDS AND SPECIFICATIONS. MIN. LAP SHALL BE ONE SPACE PLUS TWO (2) INCHES.

THE SIDES OF FOOTINGS MAY BE EARTH-FORMED IF THE EXCAVATION CAN BE KEPT VERT. CLEAN, AND STABLE, OTHERWISE, PLYWD. FORMS MUST BE USED.

4: SHOP DRAWING SUBMITTALS
SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT OR DIRECTLY TO THE ENGINEER PRIOR TO FABRICATION AND CONSTRUCTION REGARDING ALL STRUCTURAL ITEMS INCLUDING THE FOLLOWING.

CONC. MIX DESIGNS
CONC. AND MASONRY REINF.
PRE-ENGINEERED TRUSS DRAWINGS
PRE-CAST PLANK DRAWINGS
PRE-CAST STAIRS
CURTAIN WALLS
STEEL FABRICATION DRAWINGS
STEEL DETAIL DRAWINGS
STRUCTURAL STEEL CORROSION PROTECTION SPECIFICATION
OPEN WEB STEEL JOISTS
ITEMS CONSIDERED AS PART OF ASSEMBLY, I.E. WINDOWS ETC.
ANY OTHER PRE-CAST ITEMS.

DESIGN DRAWINGS, SHOP DRAWINGS AND CALCULATIONS FOR THE DESIGN AND FABRICATION OF ITEMS THAT ARE DESIGNED BY OTHERS INCLUDING PRE-CAST CONC. MEMBERS, STRUCTURAL STEEL CONNECTIONS AND PRE-MANUFACTURED WOOD TRUSSES, SHALL BE THE RESPONSIBILITY OF A DELEGATE ENGINEER. AND SHALL BEAR THE SEAL AND SIGNATURE OF THE STRUCTURAL ENGINEER REGISTERED IN THE STATE OF FLORIDA. AND SHALL BE BASED ON THE REQUIREMENTS OF THE F.B.C. THE ARCHITECT OR ENGINEER OF RECORD SHALL REVIEW THE CAPACITIES OF THE SHOP DRAWINGS DESIGN AGAINST THE STRUCTURAL LOADS AND MAKE CORRECTIONS AS REQUIRED. IF THE ENGINEER OF RECORD DOES SUPPLY THE STEEL TO STEEL CONNECTION DETAIL, THE DELEGATE ENGINEER SHALL STILL PRODUCE A SHOP DRAWINGS FOR REVIEW IF APPLICABLE.

IF THE SHOP DRAWINGS DIFFER FROM OR ADD TO THE DESIGN OF THE STRUCTURAL DRAWINGS, THEY SHALL BEAR THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF FLORIDA. ANY CHANGES TO THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ARE SUBJECT TO THE REVIEW AND ACCEPTANCE OF THE ENGINEER.

SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE W/ THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE W/ THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC. CONTRACTOR SHALL NOT BE REMOVED FROM THE RESPONSIBILITY FOR ERRORS OR OMISSIONS IN SHOP DRAWINGS OR MIX DESIGNS BY THE ENGINEER'S REVIEW.

5: CAST-IN-PLACE
REINFORCED CONC. CONSTRUCTION SHALL CONFORM TO THE F.B.C. AND A.C.I. 318 'BUILDING CODE REQUIREMENTS FOR REINF. CONC.' CONC. STRENGTHS SHALL BE VERIFIED BY STANDARD 28 DAY CYLINDER TESTS PER A.S.T.M. C39 AND SHALL BE AS FOLLOWS:

FOUNDATIONS:
fc = 3,000 P.S.I.
BEAMS, COLUMNS, ALL OTHER ITEMS UNLESS OTHERWISE SPECIFIED:
fc = 4,000 P.S.I.

CEMENT SHALL CONFORM TO A.S.T.M. C150, TYPE 1. FLY ASH CONFORMING TO A.S.T.M. C618, FOR TYPE C, MAY BE USED TO REPLACE UP TO 20% OF THE CEMENT CONTENT, PROVIDED THAT THE MIX STRENGTH IS SUBSTANTIATED BY TEST DATA COARSE AGGREGATE SHALL CONFORM TO A.S.T.M. C33 W/ A MAX. SIZE OF ¾". FINE AGGREGATE SHALL BE CLEAN, DURABLE, NATURAL SAND CONFORMING TO A.S.T.M. C33.

A WATER-REDUCING ADMIXTURE CONFORMING TO A.S.T.M. C494, USED IN STRICT ACCORDANCE W/ THE MFGR'S RECOMMENDATIONS, SHALL BE INCORPORATED IN CONC. DESIGN MIXES. A HIGH RANGE WATER-REDUCING ADMIXTURE CONFORMING TO A.S.T.M. C494, TYPE F OR G, MAY BE USED IN CONC. MIXES, PROVIDING THAT THE SLUMP DOES NOT EXCEED 8".

RESTRICT THE ADDITION OF MIX WATER AT THE JOB SITE. DO NOT ADD WATER W/O THE APPROVAL OF THE G.C. AND DO NOT EXCEED SLUMP LIMITATIONS OR TOTAL ALLOWABLE WATER TO CEMENT RATION. USE COLD WATER FROM THE TRUCK TANK AND REMIX TO ACHIEVE CONSISTENCY. THE REPORTS SHALL INDICATE HOW MUCH WATER WAS ADDED AT THE JOB SITE.

A QUALIFIED TESTING LABORATORY SHALL BE RETAINED TO PERFORM THE FOLLOWING CONCRETE TESTS ON SITE. ALL TESTS SHALL BE DONE AFTER THE ADDITION OF WATER TO THE MIX.
a. CYLINDER STRENGTH TESTS - A.S.T.N. C39; ONE SET OF FOUR CYLINDERS FOR EA. 50 CUBIC YARDS OR FRACTION THEREOF. TEST ONE CYLINDER @ 7 DAYS AND TWO @ 28 DAYS. HOLD THE FINAL CYLINDER IN RESERVE.
b. SLUMP TESTS - A.S.T.M. C143

ONE COPY OF ALL TEST REPORTS SHALL BE SENT DIRECTLY TO OWNER, ARCHITECT, STRUCTURAL ENGINEER AND G.C.

SLEEVES OPENINGS, CONDUIT AND OTHER EMBEDDED ITEMS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER BEFORE POURING. NO SLEEVE, OPENING, OR INSERT MAY BE PLACES IN BEAMS, JOISTS OR COLUMNS UNLESS APPROVED BY THE ENGINEER. CONDUITS EMBEDDED IN SLABS SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN ONE THIRD OF THE THICKNESS OF THE SLAB AND SHALL NOT BE SPACED CLOSER THAT THREE DIA. ON CENTERS.

PROVIDE ¾" CHAMFERS ON ALL EXPOSED CONC. EDGES, UNLESS NOTED OTHERWISE. WHERE INDICATED OR REQUIRED, SLOPE CONC. SLABS TO DRAINS SHOWN ON PLUMBING AND/OR ARCHITECTURAL DRAWINGS.

ALL CONC. SHALL BE CONSOLIDATED IN PLACE USING INTERNAL VIBRATORS AND CURED IMMEDIATELY AFTER FINISHING OPERATIONS.

UNDER SLAB VAPOR/MOISTURE BARRIER: STEGO 15 MIL. VAPOR BARRIER OR APPROVED EQUAL U.N.O.

CODES AND STANDARDS:
A.C.I. 301 'SPEC FOR STRUCTURAL CONC. FOR BLDGS.'
A.C.I. 305 'RECOMMENDED PRACTICE FOR NOT WEATHER CONCRETING.'
A.C.I. 318 'BLDG. CODE REQUIREMENTS FOR REINF. CONC.'
A.C.I. 315 'DETAILS AND DETAILING OF CONC. REINF.'

MIN. LAP SPLICE = 30 BAR DIA. U.N.O.

PROVIDE PROPERLY TIED SPACERS, CHAIRS, BOLSTERS, ETC. AS REQUIRED AND NECESSARY TO ASSEMBLE. PLACE AND SUPPORT ALL REINF. IN PLACE USE WIRE BAR TYPE SUPPORTS COMPLYING W/ C.R.S.I. RECOMMENDATIONS.

ALL SLABS SHALL BE POURED MONOLITHICALLY, EXCEPT FOR REQUIRED CONSTRUCTION JOINTS. PROPOSED CONSTRUCTION JOINT LOCATIONS SHALL BE SUBMITTED TO ENGINEER FOR APPROVAL.

CONTRACTOR SHALL VERIFY LOCATIONS OF ALL OPENINGS, SLEEVE AND SLAB RECESSES AS REQUIRED BY OTHER TRADES BEFORE CONC. IS PLACED.

SEE ARCHITECTURAL DRAWINGS FOR REQUIRED CONC. FINISHES.

U.N.O. SAW CUT CONSTRUCTION JOINTS ARE TO BE ¼" THE SLAB DEPTH AND SHOULD BE W/I 4 TO 12 HOURS AFTER FINISH. JOINT SPACING SHALL BE PER A.C.I. RECOMMENDATIONS UP TO A MAX. OF 15'.

A ¼" EXPANSION JOINT SHALL BE PROVIDED BETWEEN CONC. SLABS ADJOINING WALLS AND @ COLUMNS U.N.O.

SLOPE ALL BALCONIES, TERRACES AND WALKWAYS AWAY FROM THE BUILDING TO MAINTAIN POSITIVE DRAINAGE U.N.O. PROVIDE SLOPES IN SLABS WHERE FLOOR DRAINS ARE REQUIRED PER ARCHITECTURAL DRAWINGS.

6: WEATHER RESISTANCE
CONC. BALCONIES OR OTHER CONC. FLAT SURFACES EXPOSED TO THE WEATHER THROUGHOUT THE LIFE OF THE BLDG. SHALL BE TREATED W/ A CLEAR NONFLAMMABLE PENETRATING SEALER OF THE ALKYL-ALKOXY SILANE CLASSIFICATION, SUCH AS SONNEBORN PENETRATING SEALER 20, HYDROZO ENVIROSEAL 20 OR OTHER APPROVED WEATHER RESISTANCE SYSTEM. APPLICATION AND SURFACE PREPARATION SHALL BE IN ACCORDANCE W/ MFGR'S SPECIFICATIONS.

7: SHORING AND RE-SHORING
SHORING AND RE-SHORING SHALL CONFORM TO A.C.I. 347R-88. SHORING AND SUPPORTING FORMWORK SHALL NOT BE REMOVED FROM HORIZ. MEMBERS BEFORE CONC. STRENGTH IS AT LEAST 70% OF DESIGN STRENGTH. AS DETERMINED BY FIELD CURE CYLINDERS. IN ADDITION, SHORING SHALL NOT BE REMOVED SOONER THAN RECOMMENDED BY A.C.I. 347R-88, SEC. 3.7.2.3. FORMWORK SHALL NOT BE REMOVED IN LESS THAN TEN (10) DAYS. ALL SHORING SHALL BE DESIGNED BY A DELEGATE ENGINEER AND REMOVAL OF SHORING SHALL BE CONDUCTED UNDER THEIR SUPERVISION.

8: REINFORCING STEEL
REINFORCING STEEL SHALL CONFORM TO A.S.T.M. A615, GRADE 60, FOR DEFORMED BAR AND A.S.T.M. A185 FOR SMOOTH W.W.F. U.N.O. REINF. STEEL TO BE WELDED SHALL CONFORM TO A.S.T.M. A706. REINF. STEEL SHALL BE SECURELY TIED IN PLACE W/ #16 ANNEALED IRON WIRE.

ALL DETAILING AND ACCESSORIES SHALL CONFORM TO A.C.I. DETAILING MANUAL SP 66. PROVIDE CHAIRS, SPACERS, BOLSTERS AND ITEMS IN CONTACT W/ FORMS W/ HOT-DIP GALV. LEGS OR PLASTIC LEGS, ACCURATELY POSITION, SUPPORT AND SECURE REINF. AGAINST DISPLACEMENT BY FORMWORK CONSTRUCTION OR CONC. PLACEMENT OPERATIONS. 'WET-STICKING' OF REINF. IS PROHIBITED.

REQUEST CONC. COVER FOR REINF. STEEL U.N.O.
FOOTINGS 3" BOTTOM AND SIDES, 2" TOP
SLABS ¾"
COLUMNS 1½" TO TIES, 2" TOP
BEAMS 1½" TO STIRRUPS
WALLS 1½"

LAB SPLICE CONT. VERT. OR HORIZ. BARS IN CONC. MEMBERS IN ACCORDANCE W/ A.C.I. 318 LATEST EDITION, FOR CLASS 'B' TENSION LAP SPICES. DO NOT SPLICE CONT. TOP BARS IN BEAMS AT ENDS OF CLEAR SPANS. DO NOT SPLICE CONT. BOTTOM BARS IN BEAMS IN CLEAR SPANS BETWEEN SUPPORTS. SHOW ALL SPLICES ON SHOP DRAWINGS. SPLICE LOCATIONS AND METHODS SUBJECT TO APPROVAL OF STRUCTURAL ENGINEER.

AT SLAB RE-ENTRANT CORNERS, PROVIDE (2) #5x4'-0" DIAGONAL BARS. AT SLAB AND WALL OPENINGS PROVIDE A MIN. OF (2) #5 BARS ALL FOUR SIDES AND DIAGONALLY, EXTEND THESE BARS A LAP DISTANCE OR A MIN. OF 24" PAST THE OPENING OR HOOK BARS IF DISCONTINUOUS.

DOWEL ALL WALLS AND COLUMNS TO FOOTINGS W/ BAR SIZE AND SPACING TO MATCH VERT. REINF. UNLESS OTHERWISE SHOWN.
9: ADHESIVE ANCHORS
ADHESIVE ANCORS (EPOXY STYLE) SHALL HAVE THE I.C.C. E.S. OR I.A.P.M.O. U.E.S. EVALUATION REPORT INDICATE CONFORMANCE W/ CURRENT APPLICABLE I.C.C. E.S. ACCEPTANCE CRITERIA. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONC. HAVING A MIN. AGE OF 21 DAYS (A.C.I. 318-11 D.2.2.). ADHESIVE SHALL HAVE MAX. IN-SERVICE SHORT TERM TEMP. OF 150 F. AND MAX. IN-SERVICE LONG-TERM TEMP. OF 110 F. (A.C.I. 318-11 D.P.2.1). PRIOR TO INSTALLATION OF ADHESIVE ANCHORS IN HORIZ. OR UPWARDLY INCLINED ORIENTATIONS RESISTING SUSTAINED TENSION LOADS, INSTALLERS ARE REQUIRED TO BE CERTIFIED IN ACCORDANCE W/ THE A.C.I./C.R.S.I. ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM (A.C.I. 318-11 D.9.2.2) AND MUST BE CONT. INSPECTED (A.C.I. 318-11 D.9.2.4)

ACCEPTABLE ADHESIVE IN FOUNDATIONS, SLAB ON GRADE, COLUMNS AND WALLS ARE HIL TI HY-150 OR POWERS AC100+ GOLD OR SIMPSON AT-XP; IN BEAMS AND ELEVATED SLABS ARE HIL TI RE500 OR POWERS PRE1000+ OR SIMPSON SET-XP.

THREADED STUDS SHALL CONFORM TO A.S.T.M. A36 UNLESS NOTED OTHERWISE. PERMANENTLY EXPOSED STUDS SHALL BE STAINLESS STEEL NUTS AND WASHERS SHALL CONFORM TO SAME SPECIFICATIONS AS THE SUPPLIED ANCHOR RODS.

INSTALLATION SHALL BE IN CONFORMANCE W/ MFGR'S PRINTED LITERATURE. INSTALLATION SHALL ALSO INCLUDE BRUSHING AND CLEANING OF DRILLED HOLES W/ COMPRESSED AIR AS INSTRUCTED. INSTALLERS SHALL BE TRAINED BY THE MFGR'S REPRESENTATIVE. EMBEDMENT SHALL BE AS INDICATED ON THE STRUCTURAL DRAWINGS.

IDENTIFY POSITION OF REINF. STEEL AND OTHER EMBEDDED ITEMS PRIOR TO DRILLING HOLES FOR ANCHORS. EXERCISE CARE IN CORING OR DRILLING TO AVOID DAMAGING EXIST. REINF. OR EMBEDDED ITEMS. NOTIFY THE ENGINEER IF REINF. STEEL OR OTHER EMBEDDED ITEMS ARE ENCOUNTERED DURING DRILLING.

10: MASONRY
CONC. MASONRY UNITS SHALL BE A.S.T.M. C90-75, HOLLOW LOAD BEARING CONC. MASONRY UNITS, TYPE I, GRADE N-1, NORMAL WEIGHT.

MORTAR SHALL CONFORM TO A.S.T.M. C270 AND BE TYPE M OR S. SLUMP SHALL 8" TO 11".

PROVIDE DURO-O-WIRE AT 16" O.C. VERT. 9 GA. STEEL.

PROVIDE A FULL MORTAR BED ADJACENT TO GROUTED CELLS.

WHERE INDICATED ON THE DETAILS AND SEC. PROVIDE GRADE 60 REBAR IN FILLED CELLS. MAX. CELL SPACING SHALL NOT EXCEED 48" AT ANY TIME. SPACING SHOWN ON THE PLAN VIEW IS FOR ILLUSTRATION PURPOSES ONLY UNLESS SHOWN AT ADJACENT TO WALL OPENINGS.

GROUT SHALL BE 2,000 P.S.I. MIN. COMPRESSIVE STRENGTH AND MEET A.S.T.M. 476.

ALL CELLS CONTAINING VERT. BARS, BOND BEAMS AND ALL CELLS BELOW GRADE SHALL BE 48" UNLESS WITNESS HOLE OPENINGS ARE PROVIDED AT BOTTOM OF CELLS TO BE FILLED.

ALL WALLS SHALL BE 8" C.M.U. PARTIALLY REINF. MASONRY WALL W/ RUNNING BOND CONSTRUCTION W/ #5 @ 48" O.C. IN GROUT FILLED CELLS. ADD (1) #5 REINF. BAR EA. SIDE OF OPENINGS. IN OPENINGS EXCEEDING 8'-0" PROVIDE (1) #5 IN EA. OF (2) ADJACENT CELLS.

PROVIDE REINF. BARS AT CORNERS, INTERSECTIONS AND EA. SIDE OF OPENINGS. PROVIDE HOOKED DOWELS INTO FOOTINGS AND STRUCTURE ABOVE AND/OR BELOW TO PROVIDE CONTINUITY.

DO NOT PLACE CONDUITS, PIPES, ETC. IN CELLS W/ VERT. REINF. DO NOT RUN CONDUITS, PIPES, ETC. HORIZ. IN C.M.U. WALLS PARALLEL TO LENGTH OF WALL WHERE MASONRY WALLS ABUT CONC. COLUMNS TO BE PLACES PRIOR TO ERECTION OF MASONRY WALLS. PROVIDE DOVETAIL SLOTS BETWEEN COLUMN AND WALLS AND GROUT THE C.M.U. CELL CONTAINING THE DOVETAIL ANCHORS, OTHERWISE EXTEND C.M.U. HORIZ. JOINT REINF. THROUGH CONCRETE COLUMN.

IN THE EVENT A CONTRACTOR MISSES A FILLED CELL DOWEL OR HAS TO ADD ONE RETROACTIVELY THE CONTRACTOR SHALL DOWEL AND EPOXY THE MISSED REBAR A MIN. OF 12 BAR DIA. INTO THE EXIST. FOOTING AND INSTALL PER THE EPOXY SPECIFICATION PROVIDED HEREIN. THE DOWEL SHALL BE LONG ENOUGH TO PROVIDE THE REQUIRED EMBEDMENT AND THE REQUIRED LAP OF 48 BAR DIA. THEN BE SPLICED TO ANOTHER REBAR THAT IS CONT. TO THE TIE-BEAM ABOVE.

CONTROL JOINTS SHALL BE PROVIDED IN ALL CONC. MASONRY CONSTRUCTION AT LOCATIONS INDICATED ON THE ARCHITECTURAL DRAWINGS. HORIZ. WALL REINF. SHALL BE STOPPED EA. SIDE OF CONTROL JOINTS, SEE ARCHITECTURAL DRAWINGS FOR SEALANT REQUIREMENTS AT CONTROL JOINTS.

USE METAL LATH OR WIRE SCREEN FOR CAVITY CAPS. SHEET METAL, FELT, BLDG. PAPER OR LIKE MATERIALS ARE PROHIBITED.

11: TIE-BEAMS
TIE-BEAMS SHALL HAVE THE PREFIX DESIGNATION 'TB'. PROVIDE CAST-IN-PLACE CONT. TIE-BEAMS AROUND THE ENTIRE PERIMETER OF THE BLDG. U.N.O. SEE BEAM PLAN AND/OR SCH. FOR SIZE AND LOCATIONS.

TIE-BEAMS SHALL BE CONC. POURED AFTER THE BLOCK WALLS BELOW ARE IN PLACE. REINF. SHALL BE CONT. THROUGH TIE-BEAMS W/ MIN. LAP SPLICES OF 48 BAR DIA. AND BENT BARS AT CORNERS. USE METAL LATH, MORTAR OR SPECIAL UNITS TO CONFINE CONC. TO AREA REQUIRED, IN ACCORDANCE W/ A.C.I. 530.1 SEC. 3.5.B. SOLID MTL. OR FELT CAVITY CAPS ARE PROHIBITED.

12: PRE-CAST LINTELS
U.N.O. ALL LINTELS TO BE 'U' TYPE PRE-CAST CONC. UNITS EQUAL TO UNITS MFGR. BY CAST-CRETE CORP. AND PRE-STRESSED (AND ADDITIONALLY REINF. AS REQUIRED) IN ACCORDANCE W/ CAST-CRETE CORP. 'DESIGN MANUAL' LATEST EDITION, FOR THE SPAN LOADING CONDITION RELATIVE TO LINTEL LOCATION.

STATE OF FLORIDA
MICHAEL K. SHEELEY
AR0010564
REGISTERED ARCHITECT

Digitally signed by Michael K Sheeley
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No:	REVISIONS
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STRUCTURAL SPECIFICATIONS (CONT.)

LINTEL SIZE IF NOT SHOWN ON THE PLANS SHALL BE 8F16-1B FOR OPENINGS LESS THAN 8 FEET AND 8F24-1B/1T FOR OPENINGS 8'-0" TO 12'-0". PROVIDE 8" MIN. BEARING FOR LINTELS UNLESS NOTED OTHERWISE.

13. LOAD BEARING STEEL STUD FRAMING

LOAD BEARING STEEL STUD BEARING WALLS SHALL BE 'CLARK-DIETRICH' STUD S200-68 (16 GA.) 50-K.S.I. STUDS SHALL BE IN ACCORDANCE W/ AISIS 100-07 NASPEC W/ 2010 SUPPLEMENT AND I.B.C. 2012. SEE 'CLARK-DIETRICH' STRUCTURAL DESIGN GUIDE FOR ADDITIONAL INFO. STUDS ARE SIZED BASED ON PAGE 42 OF SAID MANUAL. EA. STUD SHALL BE PLACED DIRECTLY UNDER EA. LOAD BEARING TRUSS AND NOT TO EXCEED 24" O.C. AT ANY TIME. NON-LOAD BEARING WALL SHALL BE SPECIFIED BY THE ARCHITECT.

AT INT. LOAD BEARING OPENINGS JAMBS AND HEADERS SHALL BE 'CLARK-DIETRICH' REDHEADER RO. HEADERS SHALL BE 3⅞" WIDTH (16 GA.) 50-K.S.I. AND JAMBS SHALL BE 4" WIDTH 3" FLANGE (16 GA.) 50-K.S.I. HEADERS SHALL BE SIZED BY 'CLARK-DIETRICH' ENGINEERS AS PART OF THE SHOP SUBMITTAL.

HEADERS SHALL BE INSTALLED USING 'DROP N' LOCK' CLIPS, SEE PAGE 63 OF 'CLARK-DIETRICH' MANUAL FOR ADDITIONAL INFO. CLIPS AND HEADERS SHALL BE SIZED BY 'CLARK-DIETRICH' ENGINEERS AS PART OF SHOP SUBMITTAL.

ALL STEEL TO STEEL FASTENERS SHALL BE ¼"-14 H.W.H. SCREWS SPECIFIED ON PAGE 97 OF THE 'CLARK-DIETRICH' MANUAL EA. STUD TO TRACK SHALL HAVE A SIMPSON SP4 'U' SHAPED STRAP AT THE BASE WHERE STEEL TRUSSES BEAR ON STEEL STUD BEARING WALL. PROVIDE A MIN. OF A SIMPSON HTS20 PER TRUSS, OR FASTENER AS SPECIFIED ON TRUSS FRAMING PLAN.

ALL STUD TRACKS SHALL BE 16 GA. TRACK MATCHING STUD WIDTH AND SHALL BE FASTENED TO FOUNDATIONS W/ ⅝" SIMPSON TITAN H.D. 24" O.C.

MFGR'S RECOMMENDED LATERAL BRACING SHALL BE INSTALLED AT ⅓ POINTS IN THE STUDS OR AS RECOMMENDED BY MFGR.

16: PLYWOOD

PLYWD. PANELS SHALL CONFORM TO THE REQUIREMENTS OF 'U.S. PRODUCT STANDARD PS-1 FOR CONSTRUCTION AND INDUSTRIAL PLYWD. OR A.P.A. P.R.P.-108 PERFORMANCE STANDARDS U.N.O. PANELS SHALL BE A.P.A. RATED SHEATHING, EXPOSURE 1, OF THE THICKNESS AND SPAN RATING SHOWN ON THE DRAWINGS. IF NO SPECIFICATION IS GIVEN ON THE WALL SECTIONS PROVIDE ⅝" A.P.A. 32/16.

PLY WD. INSTALLATION SHALL BE IN CONFORMANCE W/ A.P.A. RECOMMENDATIONS. ALLOW ⅛" SPACING AT PANEL EDGES, UNLESS OTHERWISE RECOMMENDED BY THE PANEL MFGR.

ALL ROOF AND SHEAR WALL SHEATHING SHALL BE INSTALLED W/ FACE GRAIN PERPENDICULAR TO SUPPORTS, EXCEPT AS INDICATED ON THE DRAWINGS. STAGGER ENDS OF ADJACENT PANELS 4'-0".

ROOF SHEATHING SHALL BE BLOCKED, TONGUE AND GROOVE, OR HAVE EDGES SUPPORTED BY PLY CLIPS. ATTACH PLYWD. PANELS TO SUPPORTING MEMBERS W/ 8d RING SHANK NAILS SPACED 4" O.C. ALONG THE PANEL EDGES AND AT 6" O.C. ALONG INTERMEDIATE SUPPORTS U.N.O.

SHEAR WALL SHEATHING SHALL BE BLOCKED W/ 2x FRAMING AT ALL PANEL EDGES. ATTACH PLYWD. PANELS TO SUPPORTING MEMBERS W/ 8d NAILS SPACED 4" O.C. ALONG INTERMEDIATE SUPPORTS U.N.O.

ATTACH PLYWD. PANELS TO SUPPORTING MEMBERS W/ 8d NAILS SPACES 4" O.C. ALONG THE PANEL EDGES AND AT 6" O.C. ALONG INTERMEDIATE SUPPORTS.

17: EXPANSION AND SCREW ANCHORS

EXPANSION AND SCREW ANCHORS SHALL HAVE THE I.C.C. E.S. OR I.A.P.M.O. USE EVALUATION REPORT INDICATED CONFORMANCE W/ CURRENT APPLICABLE I.C.C. E.S. ACCEPTANCE CRITERIA.

ACCEPTABLE EXPANSION ANCHORS ARE HILTI KWIK-BOLT TZ. POWERS SD2 OR SIMPSON STRONG BOLT 2.

ALL WEDGE ANCHORS 'REDHEADS' SIMPSON TITAN 'TAPCON' OR SIMPSON TITAN H.D. BOLTS SHALL BE INSTALLED PER THE MFGR. GUIDELINES AND SPACING. EDGE DISTANCE AND EMBEDMENT, AS WELL AS INSTALLATION AND PREPARATION INSTRUCTIONS.

'TAPCONS' WHERE SPECIFIED SHALL BE ¼" DIA. SIMPSON TITAN SCREW W/ 1¾" EMBEDMENT. EDGE SPACING 1½" MIN. AND A SPACING OF 3".

'SIMPSON TITAN H.D.' OR 'L.D.T.' WHERE SPECIFIED SHALL BE AS FOLLOWS:

WHERE SPECIFIED AS ½" DIA. SHALL HAVE 4" EMBEDMENT U.N.O.:
EDGE DISTANCE OF 4½". SPACING OF 6" MIN. CONC. THICKNESS OF 10"

WHERE SPECIFIED AS ⅝" DIA. SHALL HAVE 5½" EMBEDMENT U.N.O.:
EDGE DISTANCE OF 5½" SPACING OF 6" CONC. THICKNESS OF 10"

WHERE SPECIFIED AS ¾" DIA. SHALL HAVE 6¼" EMBEDMENT U.N.O.:
EDGE DISTANCE OF 7⅝" SPACING OF 6" CONC. THICKNESS OF 10"

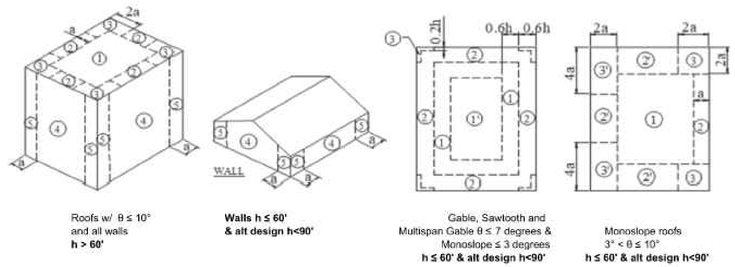
'REDHEAD' WHERE SPECIFIED SHALL BE AS FOLLOWS:

WHERE SPECIFIED AS ½" DIA. SHALL HAVE:
3⅜" EMBEDMENT INTO CONC. 5" CRITICAL EDGE DISTANCE, 6" CRITICAL SPACING, CONC. THICKNESS 1½ TIMES EMBEDMENT.

WHERE SPECIFIED AS ⅝" DIA. SHALL HAVE:
4½ EMBEDMENT INTO CONC. 6¼" CRITICAL EDGE DISTANCE, 6¼" CRITICAL SPACING, CONC. THICKNESS 1½ TIMES EMBEDMENT.

WHERE SPECIFIED AS ¾" DIA. SHALL HAVE:
5" EMBEDMENT INTO CONC. 7½" CRITICAL EDGE DISTANCE, 7" CRITICAL SPACING, CONC. THICKNESS 1½ TIMES EMBEDMENT.

Location of C&C Wind Pressure Zones - ASCE 7-16

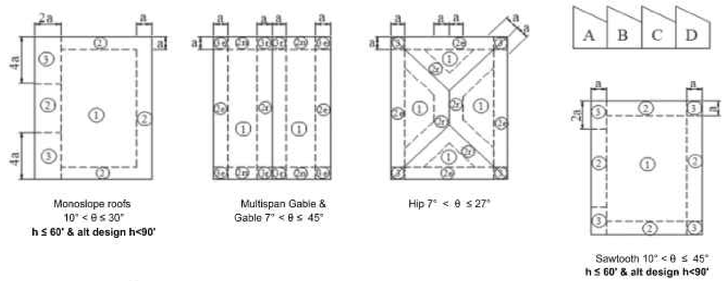


Roofs w/ θ ≤ 10° and all walls
h > 60'

Walls h ≤ 60' & alt design h<90°

Gable, Sawtooth and Multispan Gable θ ≤ 7 degrees & Monoslope ≤ 3 degrees
h ≤ 60' & alt design h<90°

Monoslope roofs 3° < θ ≤ 10°
h ≤ 60' & alt design h<90°

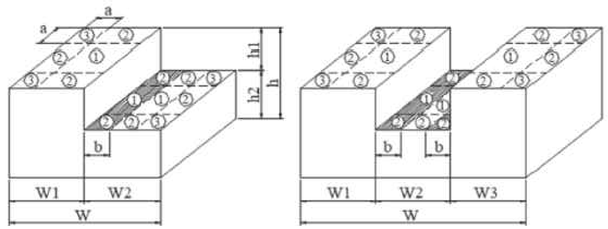


Monoslope roofs 10° < θ ≤ 30°
h ≤ 60' & alt design h<90°

Multispan Gable & Gable 7° < θ ≤ 45°

Hip 7° < θ ≤ 27°

Sawtooth 10° < θ ≤ 45°
h ≤ 60' & alt design h<90°



Stepped roofs θ ≤ 3°
h ≤ 60' & alt design h<90°

Wind Loads - Components & Cladding : h ≤ 60'

Kh (case 2) =	0.59	h =	16.0 ft
Base pressure (qh) =	**22.2 psf**	a =	3.0 ft
Minimum parapet ht =	0.0 ft	GCpi =	+/-0.18
Roof Angle (θ) =	7.1 deg	ql = qh =	22.2 psf
Type of roof =	Monoslope		

Roof

	GCp +/- GCpi				Surface Pressure (psf)			
Area	10 sf	20 sf	50 sf	100 sf	10 sf	20 sf	50 sf	100 sf
Negative Zone 1	-1.28	-1.28	-1.28	-1.28	-28.4	-28.4	-28.4	-28.4
Negative Zone 2	-1.48	-1.45	-1.41	-1.38	-32.8	-32.2	-31.3	-30.6
Negative Zone 2'	-1.78	-1.75	-1.71	-1.68	-39.5	-38.8	-38.0	-37.3
Negative Zone 3	-1.98	-1.8	-1.56	-1.38	-43.9	-39.9	-34.6	-30.6
Negative Zone 3'	-2.78	-2.48	-2.08	-1.78	-61.7	-55.0	-46.2	-39.5
Positive All Zones	0.48	0.45	0.41	0.38	16.0	16.0	16.0	16.0

Walls

	GCp +/- GCpi				Surface Pressure at h			
Area	10 sf	100 sf	200 sf	500 sf	10 sf	100 sf	200 sf	500 sf
Negative Zone 4	-1.17	-1.01	-0.96	-0.90	-26.0	-22.4	-21.4	-20.0
Negative Zone 5	-1.44	-1.12	-1.03	-0.90	-32.0	-24.9	-22.8	-20.0
Positive Zone 4 & 5	1.08	0.92	0.87	0.81	24.0	20.4	19.4	18.0

Note: GCp reduced by 10% due to roof angle <= 10 deg.

LOAD DESIGN CRITERIA:
DESIGN WAS BASED ON STRENGTH AND DEFLECTION CRITERIA OF THE 2023 F.B.C. IN ADDITION TO THE DEAD LOADS, THE FOLLOWING LOADS AND ALLOWABLES WERE USED FOR DESIGN, W/ LIVE LOADS REDUCED PER THE 2023 F.B.C.

ROOF: 20 PSF LL / 20 PSF SDL

DESIGN PARAMETERS:

APPLICABLE CODES:
BUILDING CODES = FLORIDA BUILDING CODES, BUILDING 8th EDITION 2023
MECHANICAL CODE = FLORIDA BUILDING CODE, MECHANICAL 2023
PLUMBING CODE = FLORIDA BUILDING CODE, PLUMBING 2023
FUEL GAS CODE = FLORIDA BUILDING CODE, FUEL GAS 2023
ELECTRICAL CODE = 2020 NATIONAL ELECTRIC CODE
FFPC = 2023 FLORIDA FIRE PREVENTION CODE
ACCESSIBILITY CODE = FLORIDA BUILDING CODE, BUILDING 2023
ENERGY CODE = FLORIDA BUILDING CODE, BUILDING 2023

METHOD OF DESIGN:
DESIGNED PURSUANT TO FLORIDA BUILDING CODE CHAP. 16, BUILDING EIGHTH EDITION 2023

IMPORTANCE FACTOR:
☐ BUILDING CATEGORY I ☐ BUILDING CATEGORY III
☒ BUILDING CATEGORY II ☐ BUILDING CATEGORY IV

BUILDING OCCUPANCY CLASSIFICATION:
☐ GROUP A - ASSEMBLY ☐ GROUP H - HAZARDOUS
☒ GROUP B - BUSINESS ☐ GROUP I - INSTITUTIONAL
☐ GROUP D - DAY CARE CENTER ☐ GROUP M - MERCANTILE
☐ GROUP E - EDUCATIONAL ☐ GROUP R - RESIDENTIAL
☐ GROUP F - FACTORY INDUSTRIAL ☐ GROUP S - STORAGE

BASIC WIND SPEED:
☒ 170 MPH (ULTIMATE)

BUILDING CONSTRUCTION TYPE:
☐ TYPE I-A ☐ TYPE II-B ☐ TYPE IV
☐ TYPE I-B ☐ TYPE III-A ☐ TYPE V-A
☐ TYPE II-A ☐ TYPE III-B ☒ TYPE V-B

EXPOSURE CATEGORY:
☐ A ☐ C
☒ B ☐ D

WINDBORNE DEBRIS REGION:
☐ NO
☒ YES
☒ IMPACT RESISTANT GLASS
☐ IMPACT RESISTANT SHUTTERS
☐ IMPACT RESISTANT STORM PANELS
☐ IMPACT RESISTANT COMBINATION OF GLASS & STORM PANELS

INTERNAL PRESSURE COEFFICIENTS:
☐ 0.00 (OPEN)
☒ +0.18, -0.18 (ENCLOSED)
☐ +0.55, -0.55, (PARTIALLY ENCLOSED)

No:	REVISIONS
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AR0010564
REGISTERED ARCHITECT

JOB:	MK25015B
DATE:	09-02-2025
DRWN. BY:	CR
CAD REF:	STRUC

SHEET
S2

ROOF / FRAMING NOTES:

1. REFER TO ADDITIONAL DRAWINGS FOR CEILING HEIGHT AND SLOPE OVERHANG DIMENSIONS AND HEAD HEIGHT.
2. ROOF PLAN FOR DESIGN PURPOSES ONLY. DESIGN ROOF FOR WIND LOAD OF 170 MPH WIND SPEED, EXPOSURE B, IMPORTANCE FACTOR 1.0, CATEGORY (II) PER THE 2023 FLORIDA BUILDING CODE. USE 10 PSF DEAD LOAD MAXIMUM TO RESIST UPLIFT.
3. TYPICAL ROOF FRAMING TIE DOWN SIMPSON HETA16 UNLESS NOTED OTHERWISE (U.N.O.).
4. ALL STRUCTURAL AND EXTERIOR FRAMING LUMBER SHALL BE SOUTHERN PINE GRADE NO. 2 OR BETTER.
5. ALL WOOD MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH MASONRY, CONCRETE OR SOIL, SHALL BE PRESSURE TREATED.
6. CONTRACTOR SHALL PROVIDE ALL FASTENING DEVICES NECESSARY AND SUITED FOR EACH APPLICATION. FASTENINGS SUBJECT TO MOISTURE SHALL BE HOT DIPPED GALVANIZED TO ASTM A-153-80.
7. ALL EXTERIOR ROOF SHEATHING SHALL BE GRADE APA RATED 5/8" AND WALL SHEATHING SHALL BE EXTERIOR GRADE APA RATED 1/2" MINIMUM. FASTEN WITH #10, 1-3/4" LONG TEK SCREWS AT 6" O.C. AT EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS. SEE ROOF UPLIFT SCHEMATIC FOR ROOF SHEATHING CONNECTORS.

ROOF SHEATHING CONNECTORS

MINIMUM CONNECTIONS PER PLYWOOD SHEET

PERIMETER OF EA. SHEET - #10, 1-3/4" LONG TEK SCREWS @ 6" o.c.
FIELD OF EACH SHEET - #10, 1-3/4" LONG TEK SCREWS @ 12" o.c.

MINIMUM CONNECTIONS PER UPLIFT ZONE

(1) -GENERAL UPLIFT ZONES - #10, 1-3/4" LONG TEK SCREWS @ 6" o.c. PERIMETER
#10, 1-3/4" LONG TEK SCREWS @ 12" o.c. FIELD

(2) -END ZONES - #10, 1-3/4" LONG TEK SCREWS @ 6" o.c. PERIMETER
#10, 1-3/4" LONG TEK SCREWS @ 6" o.c. FIELD

(2) -OVERHANG ZONES - #10, 1-3/4" LONG TEK SCREWS @ 6" o.c. PERIMETER
#10, 1-3/4" LONG TEK SCREWS @ 6" o.c. FIELD

(2) -COVERED TERRACE ZONES - #10, 1-3/4" LONG TEK SCREWS @ 6" o.c. PERIMETER
#10, 1-3/4" LONG TEK SCREWS @ 6" o.c. FIELD

(3) -CORNER OVERHANG ZONES - #10, 1-3/4" LONG TEK SCREWS @ 6" o.c. PERIMETER

SEE STRUCTURAL NOTES PLYWOOD GRADES REQUIRED.

BEAM SCHEDULE

BEAM DESIGNATION	WIDTH	HEIGHT	REINFORCEMENT	TOP OF BEAM ELEVATION	NOTES
B-1	8"	8"	5,000 PSI CONCRETE. #3 TIES AT 3" O.C. AND (2) #5 BARS BOTTOM AND (2) #6 BARS TOP	SLOPED, VARIES	CAST-IN-PLACE CONC. BEAM
B-2	8"	1'-4"	(2) #5 BARS TOP AND BOTTOM AND #3 TIES AT 6" O.C.	SLOPED, VARIES	CAST-IN-PLACE CONC. BEAM
B-3	4"	8"	BY PRECAST MANUFACTURER	(+) 3'-4"	4" U-BLOCK BELOW STOREFRONT

NOTE:
SIZE ALL PRECAST LINTELS PER
WINDOW MANUFACTURERS SPECS.

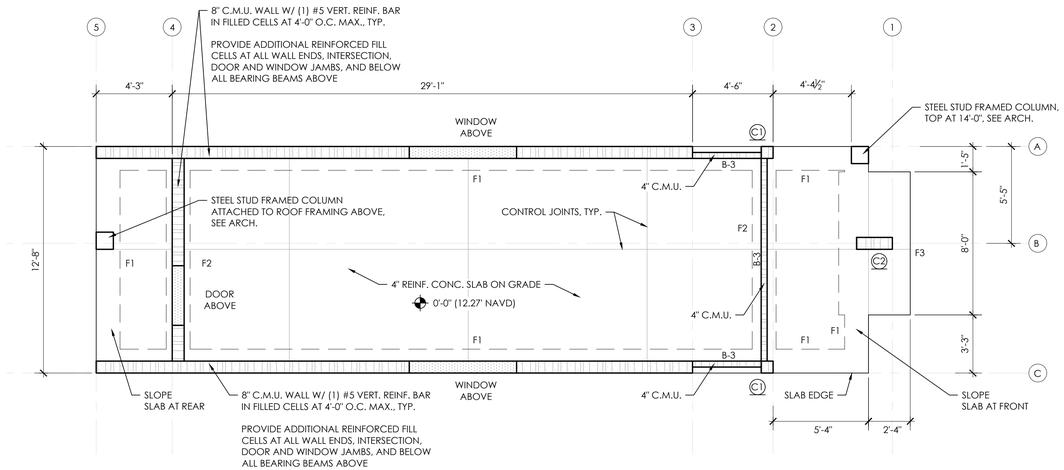
FOOTING SCHEDULE

FOOTING DESIGNATION	SIZE W	SIZE L	SIZE D	REINFORCEMENT
F1	1'-4"	CONT.	1'-4"	(3) #5 BOTTOM, CONT.
F2	1'-4"	CONT.	1'-4"	(3) #5 BOTTOM, CONT.
F3	4'-0"	8'-0"	4'-0"	(7) #5 BOTTOM L.W., #5 AT 12" O.C. S.W.

NOTE:
REINFORCEMENT MIN. 3" CLEAR FROM ALL SIDES.

COLUMN SCHEDULE

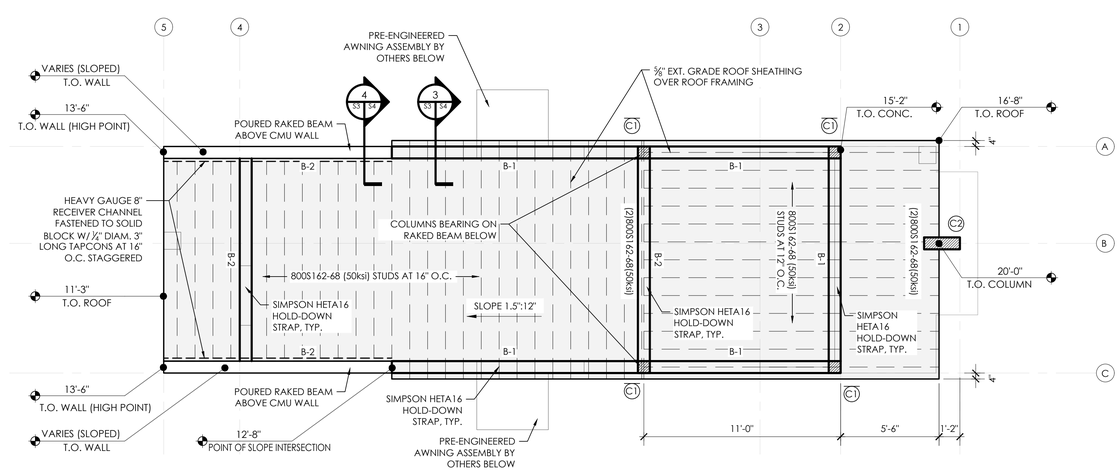
COLUMN	COLUMN DESCRIPTION
C1	8x8 COLUMN COLUMN W/ (4) #5 BARS VERT. & #3 TIES AT 8" O.C.
C2	8x24 CONCRETE COLUMN, REINFORCED W/ (6) #7 BARS & #3 STIRRUPS AT 8" O.C. VERTICALLY



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

0 4' 8' 12'



ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

0 4' 8' 12'

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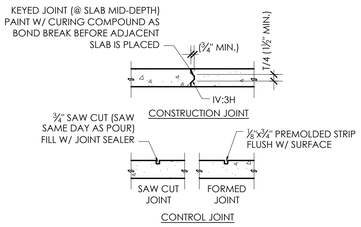
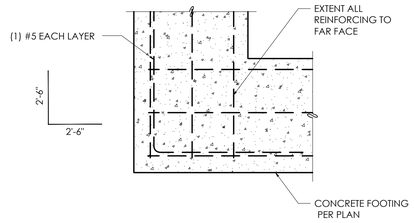


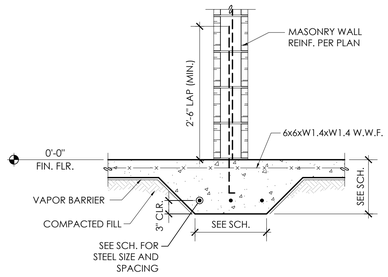
JOINT DETAILS (TYP.)

SCALE: N.T.S.



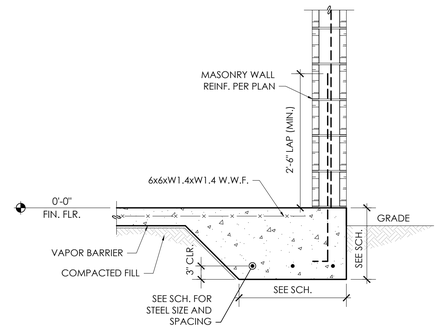
CORNER FOOTING DETAIL (TYP.)

SCALE: N.T.S.



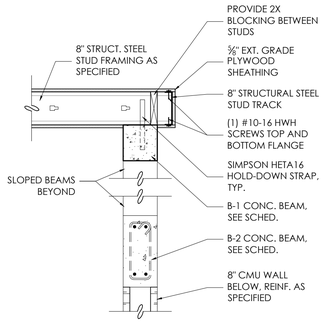
2/S4 F2 FOOTING DETAIL

SCALE: 3/4" = 1'-0"



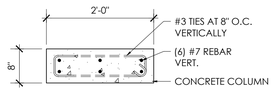
1/S4 F1 FOOTING DETAIL

SCALE: 3/4" = 1'-0"



3/S4 ROOF FRAMING CONNECTION DETAIL

SCALE: 3/4" = 1'-0"



C2 COLUMN DETAIL

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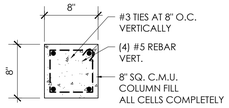

C1 COLUMN DETAIL

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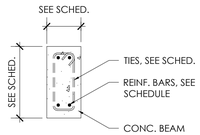

POURED CONC. BEAM DETAIL (TYP.)

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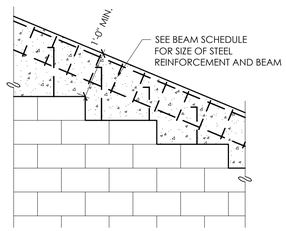

RAKED BEAM DETAIL (TYP.)

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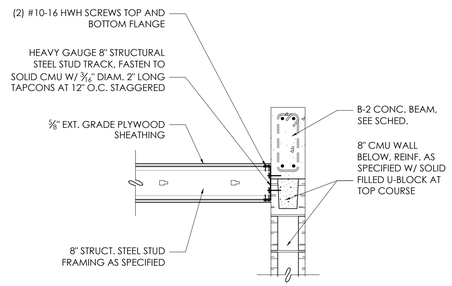
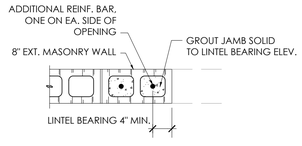


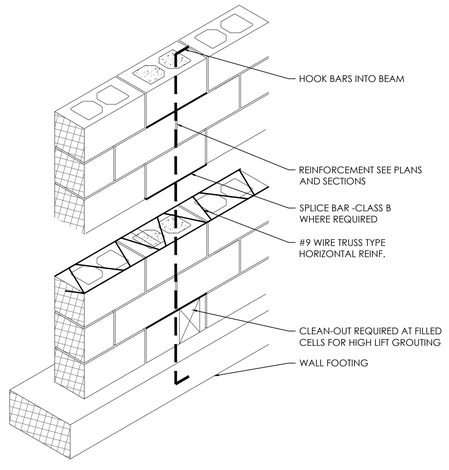
4/S4 ROOF FRAMING CONNECTION DETAIL

SCALE: 3/4" = 1'-0"



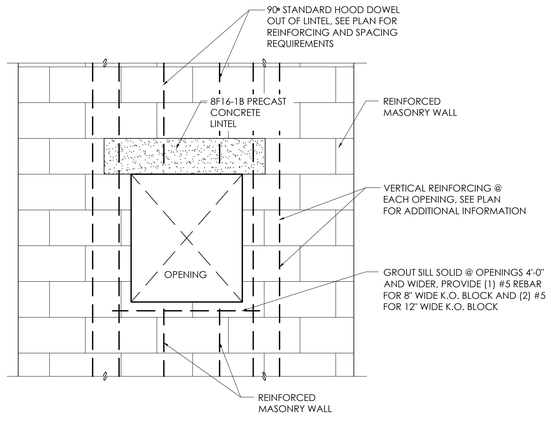
C.M.U. WALL OPENING DETAIL (TYP.)

SCALE: N.T.S.



TYP. MASONRY CONSTRUCTION DETAIL

SCALE: N.T.S.



CONC. LINTEL/OPENING REINF.

SCALE: N.T.S.

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